



### **TEST REPORT**

Rendered to:

**COLOR GUARD, INC.** 

For:

Lincoln Aluminum Guardrail System

Report No.: F8799.02-119-19

**Report Date:** 09/02/16

Test Record Retention Date: 07/21/20





### **TEST REPORT**

F8799.02-119-19 September 2, 2016

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### **TEST REPORT**

Rendered to:

COLOR GUARD, INC. 320 Range Line Road Sheboygan Falls, Wisconsin 53085

> Report No.: F8799.02-119-19 Test Dates: 07/06/16

Through: 07/21/16

Report Date: 09/02/16

Test Record Retention Date: 07/21/20

### 1.0 General Information

### 1.1 Product

Lincoln Aluminum Guardrail System

### 1.2 Project Description

Architectural Testing, Inc., an Intertek company ("Intertek-ATI"), was contracted by Color Guard, Inc. to perform material and structural testing on their 6 ft by 42 in and 8 ft by 42 in *Lincoln Aluminum* guardrail systems. This report is in conjunction with Intertek-ATI Report No. F8799.03-117-38 which includes product sampling information. The purpose of the testing is code compliance evaluation in accordance with the following criteria:

ICC-ES™ AC273 (March 1, 2008 - Editorial Revised March 2016), Acceptance Criteria for Handrails and Guards

ICC-ES™ AC273-08 was developed by the ICC Evaluation Service, Inc. (ICC-ES™) as acceptance criteria to evaluate compliance with the following building codes:

2015 International Building Code®, International Code Council

2015 International Residential Code®, International Code Council

### 1.3 Limitations

All tests performed were to evaluate structural performance of the railing assembly to carry and transfer imposed loads to the supports (posts). The test specimen evaluated included the pickets, rails, rail brackets, posts, and attachment to the supporting structure. Anchorage of support posts to the supporting structure is not included in the scope of this testing and would need to be evaluated separately.

Testing is limited to satisfying the IRC - One- and Two-Family Dwellings requirements of ICC-ES<sup>™</sup> AC273.





### 1.4 Qualifications

Intertek-ATI has demonstrated compliance with ISO/IEC International Standard 17025 and is consequently accredited as a Testing Laboratory (TL-144) by International Accreditation Service, Inc. (IAS).

### 1.5 Product Description

The *Lincoln Aluminum* guardrail system is comprised of aluminum rails, pickets, and post sleeves as well as steel post mounts. Test specimens consisted of one product color: Black. Drawings are included in Appendix A to verify the overall dimensions and other pertinent information of the tested product, its components, and any constructed assemblies.

### 1.6 Product Sampling

A representative of Intertek-ATI visited the Poly Vinyl Company facility in Sheboygan Falls, Wisconsin, on 06/17/16 to select the components used for testing. All samples selected for testing were marked for identification and were the samples used for all tests reported herein. Reference Intertek-ATI report No. F8799.03-117-38 for additional product sampling information. See photograph in Appendix B for typical sampling mark.

### 1.7 Witnessing

Tad Rauber from Color Guard, Inc. and Robin McCoy from Poly Vinyl Company were present on 07/06/16 and 07/07/16 for testing conducted and reported herein.

### 1.8 Conditions of Testing

Unless otherwise indicated, all testing reported herein was conducted in a laboratory set to maintain temperature in the range of  $68 \pm 4^{\circ}F$  and humidity in the range of  $50 \pm 5\%$  RH.

### 2.0 Referenced Standards

ASTM B221-14, Standard Specification for Aluminum and Aluminum-Alloy Extruded Bars, Rods, Wire, Profiles and Tubes

ASTM D1761-12, Standard Test Methods for Mechanical Fasteners in Wood

ASTM E8/E8M-15, Standard Test Methods for Tension Testing of Metallic Materials





### 3.0 Tensile Testing

Re: ICC-ES™ AC273 - Section 4.1

### 3.1 General

Tensile tests were performed on major railing components (top rail) used in the railing assemblies for the purpose of verifying the material specification.

### 3.2 Test Specimens

One set of fifteen test specimens was taken from tested top rails.

### 3.3 Test Procedure

The specimens were tested using an Instron® Model 3369 Universal Testing Machine with SATEC "T" grips and operating at a uniform cross-head speed of 0.2 in/min. Strain was measured using a SATEC Model T1M snap-on Extensometer with a 2 in gage length. See photograph in Appendix B. Testing was conducted on 07/21/16.

### 3.4 Test Results

### Lincoln Aluminum - Top Rail

Sample ID	Width (in)	Thickness (in)	MOE (10 <sup>6</sup> psi)	Yield (10³ psi)	Tensile (10³ psi)	Elongation (%)	Max Load (lbf)
1	0.497	0.087	9.71	26.9	31.6	16.2	1368
2	0.499	0.086	10.01	27.0	31.7	16.4	1367
3	0.499	0.087	9.54	27.3	32.0	15.8	1394
4	0.499	0.087	9.96	27.4	32.2	16.6	1397
5	0.500	0.087	9.50	26.8	31.5	17.0	1368
6	0.500	0.087	9.58	27.2	31.8	16.4	1382
7	0.500	0.087	9.41	27.6	32.2	16.7	1406
8	0.502	0.087	9.87	27.7	32.3	15.2	1411
9	0.500	0.087	10.01	27.5	32.0	16.8	1388
10	0.500	0.087	9.71	27.8	32.3	16.7	1409
11	0.499	0.083	9.56	28.3	32.0	15.2	1323
12	0.498	0.087	9.85	28.2	32.1	16.8	1391
13	0.500	0.084	9.65	28.1	31.8	13.9	1328
14	0.503	0.087	9.73	28.0	31.9	14.8	1393
15	0.502	0.084	9.73	27.8	31.7	15.2	1331
Minimum:			9.41	26.9	31.5	13.9	1323
		Maximum:	10.01	28.3	32.3	17.0	1411
		Mean:	9.7	27.6	32.0	16.0	1377
	Standard	d Deviation:	0.2	0.5	0.3	0.9	29.3
Coe	Coefficient of Variation:			1.7%	0.8%	5.7%	2.1%





### 3.5 Analysis of Test Results

Per Color Guard, Inc., the material used in their guardrail system tested and reported herein was specified as ASTM 6063-T5.

The following criteria are listed under ASTM B 221, Table 2 - *Mechanical Property Limits,* for 6063-T5:

- Tensile strength, min, psi 22,000
- Yield strength, min, psi 16,000

	Min. Tensile Strength (psi)	Min. Yield Strength (psi)
6063-T5	22,000	16,000
Top Rail Samples	31,951	27,573

### 4.0 Assembly Fastener Testing

Re: ICC-ES™ AC273 - Section 4.2.7

### 4.1 General

The purpose of this testing was to simulate a 90 degree bracket loading condition, which addresses a situation when the guardrail system is to be installed with the top rails in a corner condition.

### 4.2 Test Specimens

Short sections of the top rail were attached in accordance with Color Guard Inc.'s installation instructions to short sections of posts. Specimens were assembled by an Intertek-ATI technician. Rail brackets were secured to the post and to the rail as described in Section 5.4 Fastening Schedule.

### 4.3 Test Setup

The testing machine was fitted with the post sections at the top and bottom to accommodate anchorage of the rail and brackets. The top post section was attached to the test machine's crosshead with a swivel mechanism, and the bottom post section was attached rigidly to the base of the test machine. Three specimens were tested in this manner with each of the three specimens including two connections for a total of six connections. See photograph in Appendix B for test setup.

### 4.4 Test Procedure

Testing was performed in accordance with ASTM D 1761 and by using a computer-monitored and -controlled SATEC Unidrive, Model MII 50 UD Universal Testing Machine. Tests were run at a crosshead speed of 0.05 in/min, and each specimen was tested in tension to its ultimate load capacity. Testing was conducted on 07/07/16.





### 4.5 Test Results

Sample No.	Ultimate Load (lb)	Deviation From Average	Mode of Failure
1	552	-8.9%	
2	617	+1.8%	Bracket failure
3	650	+7.3%	
Average	606		
Allowable Capacity 1	202	≥ 200 lb ∴ OK	

<sup>&</sup>lt;sup>1</sup> Average ultimate load divided by a factor of safety of three (3.0)

### 4.6 Summary and Conclusions

The maximum design load rating required for guardrail systems for use in IRC - One- and Two-Family Dwellings and for rail lengths up to and including 8 ft for use in IBC - All Use Groups is 200 lb. Therefore, fasteners / connectors reported herein meet the performance requirements of ICC-ES™ AC273 for use in corner conditions.

### 5.0 Structural Performance Testing of Assembled Railing Systems

Re: ICC-ES™ AC273 - Section 4.2.1

### 5.1 General

Railing assemblies were tested in a self-contained structural frame designed to accommodate anchorage of a rail assembly and application of the required test loads. The specimen was loaded using an electric winch mounted to a rigid steel test frame. High strength steel cables, nylon straps, and load distribution beams were used to impose test loads on the specimen. Applied load was measured using an electronic load cell located in-line with the loading system. Deflections were measured to the nearest 0.01 in using electronic linear displacement transducers.

### 5.2 Railing Assembly Description

The Lincoln Aluminum guardrail systems consisted of aluminum top and bottom rails with spaced pickets between the rail members. The railing systems had an overall top rail length (inside of post to inside of post) of 72 in and 96 in with an overall rail height (top of top rail to bottom of bottom rail) of 40-1/4 in. Top and bottom rails attached to aluminum post sleeves via aluminum socket brackets. Two support blocks were equally spaced along the bottom rail for the 96 in guardrails and one support block was located at the midspan of the bottom rail for the 72 in guardrail. See Section 5.4 Fastening Schedule for connection details. See drawings in Appendix A and photographs in Appendix B for additional details.





### 5.3 Series / Model

The test specimen components were supplied by Color Guard, Inc. and were assembled by their representatives, Tad Rauber and Robin McCoy.

<u>Top Rail</u>: 2-1/16 in high by 2-5/16 in wide by 0.09 in wall extruded 6063-T5 aluminum "breadloaf" profile

Bottom Rail: 1-1/4 in high by 1-1/2 in wide by 0.10 in wall extruded 6063-T5 aluminum

Top Rail Brackets: 2-1/4 in high by 2-1/2 in wide by 1-1/4 in deep cast aluminum socket

bracket (0.08 in wall)

Bottom Rail Brackets: 1-15/16 in high by 1-3/4 in wide by 1-1/4 in deep cast aluminum

socket bracket (0.09 in wall)

Aluminum Rail Insert: 1/4 in high by 1-1/8 in wide by 0.09 in thick extruded 6063-T5

aluminum insert with routings for pickets

PVC Rail Insert: 15/16 in high by 1-5/16 in wide by 0.04 in wall vinyl insert

Pickets: 3/4 in square by 0.04 in wall extruded 6063-T5 aluminum

Post: 2 in O.D. by 0.11 in thick powder-coated steel tube post welded to nominal 3-1/2 in square, nominal 1/4 in thick base plate with four nominal 7/16 in diameter holes located approximately 1/2 in on-center in from each edge and approximately 2-1/2 in apart on-center, four 5/16 dia. holes located approximately 3/8 in on-center in from edge and approximately 0.55 in from the centerline of the tube on each side of the plate, and one 0.56 in diameter hole located in the center of the base plate - the continuous fillet weld connecting the tube to the base plate was 3/16 in - the base plate was attached to a rigid steel surface either directly to the surface (simulated concrete), or to 5/4 decking and 2x8 pressure treated lumber (surface mount wood) as described in Section 5.4 Fastening Schedule. A 3-1/2 in square by 0.12 in thick leveling plate (with hole pattern matching the base plate) was utilized for simulated concrete installation testing only.

<u>Post Spacer</u>: 2-11/16 in wide by 7/8 in deep by 4 in long extruded PVC – one spacer is attached on each side of the post at 3-1/2 in and 41-1/2 in from the deck surface to the center of spacer with connection details as described in Section 5.4 Fastening Schedule.

Post Sleeve: 3 in square by 0.12 in wall extruded 6063-T5 aluminum

Support Block: 3/4 in square by 0.04 in wall extruded 6063-T5 aluminum picket cut to

length and secured with a PVC baluster plug to the bottom rail as described  $\,$ 

in Section 5.4 Fastening Schedule.

Baluster Plug: 5/8 in dia. by 3/4 in long vinyl plug with a 3/16 dia. hole for attached as

described in Section 5.4 Fastening Schedule

See drawings in Appendix A and photographs in Appendix B for additional details.





### 5.4 Fastening Schedule

Connection	Fastener
Rail Bracket to Post*	Two #10-16 x 1" (0.137 in minor dia.) pan-head, square-drive, self-drilling, stainless steel screws
Top Rail Bracket to Rail*	One #10 x 1 in long (0.137 in minor dia.) pan-head, square-drive, self-drilling, stainless steel screw
Bottom Rail Bracket to Rail	Slip fit - No mechanical connection
Baluster to Rail	Slip III - No mechanical connection
Rail Insert to Rail	Channel fit
Foot Block to Baluster Plug	Slip fit - No mechanical connection
Baluster Plug to Bottom Rail	One #10-16 x 1" (0.137 in minor dia.) pan-head, square-drive, self-drilling stainless steel screw
PVC Spacer to Post	One #1/4-14 x 1" (0.185 in minor dia.), pan head phillips- drive, self-drilling, galvanized steel screw
Post Mount to Wood Surface Mount	Four 2/8 in Grado 5 how hoad holts with put and washer
Post Mount to Steel Channel (simulated concrete installation)	Four 3/8 in Grade 5 hex-head bolts with nut and washer with bottom of base plate set 1/8 in off leveling plate

<sup>\* 7/32</sup> in diameter pre-drill used

### 5.5 Test Setup

The railing assembly was installed and tested as a single railing section by directly securing (surface-mounting) the base of the post mounts to a rigid steel test frame. The railing was assembled by an Intertek-ATI technician with assistance from Color Guard, Inc. representatives, Tad Rauber and Robin McCoy. Transducers mounted to an independent reference frame were located to record movement of reference points on the railing system components (ends and mid-point) to determine net component deflections. See photographs in Appendix B for test setups.





### 5.6 Test Procedure

Testing and evaluation was performed in accordance with Section 4.2.1 of ICC-ES<sup>™</sup> AC273. The test specimen was inspected prior to testing to verify size and general condition of the materials, assembly, and installation. No potentially compromising defects were observed. One specimen was used for all load tests which were performed in the order reported. Each design load test was performed using the following procedure:

- 1. Zeroed transducers and load cell at zero load;
- 2. Increased load to specified test load in no less than ten seconds; and
- 3. Held test load for no less than one minute.

### 5.7 Test Results

Unless otherwise noted, all loads and displacement measurements were normal to the rail (horizontal). The test results apply only to the railing assembly between supports and anchorage to the support.

### **Key to Test Results Tables:**

Load Level: Target test load

<u>Test Load</u>: Actual applied load at the designated load level (target). Where more than one value is reported, the test load was the range (min. - max.) that was held during the time indicated in the test.

<u>Elapsed Time (E.T.)</u>: The amount of time into the test with zero established at the beginning of the loading procedure. Where more than one value is reported, the time was the range (start-end) that the designated load level was reached and sustained.





### Series No. 1

## 96 in by 42 in *Lincoln* Railing with Structural Posts on Surface Mount Wood Installation Limited to Use in IRC - One- and Two-Family Dwellings / ICC-ES™ AC273

### Specimen No. 1 of 3

	Test No. 1 - Test Date: 07/07/16						
Design	Load: 50 lb / 1	Square ft of In-Fi	II at Center of Two Pickets				
Load Level Test Load E.T.			Result				
Loau Level	(lb)	(min:sec)	nesuit				
125 lb			Sustained load equal to or greater				
(2.50 x D.L.)	125 - 132	00:14 - 01:23	than 125 lb for one full minute				
(2.50 X D.L.)			without failure				

Test No. 2 - Test Date: 07/07/16  Design Load: 50 lb / 1 Square ft of In-Fill at Bottom of Two Pickets						
Load Level Test Load E.T. (lb) (min:sec)			Result			
125 lb (2.50 x D.L.)	125 - 129	00:26 - 01:34	Sustained load equal to or greater than 125 lb for one full minute without failure			

Test No. 3 - Test Date: 07/07/16  Design Load: 200 lb Concentrated Load at Mid-Span of Top Rail						
Load Lovel	Test Load	E.T.		Displace	ment (in)	
Load Level	(lb)	(min:sec)	End	Mid	End	Net 1
200 lb (D.L.)	202	00:28	0.49	1.40	0.51	0.90
500 lb (2.50 x D.L.)	500 - 503	00:47 - 01:53		han 500 lb	d load equ for one fu t failure	

### **Deflection Evaluation:**

Maximum rail deflection at 202 lb = 0.90 in on an 8 ft rail

$$\text{Limits per AC273 $^2$:} \left(\frac{h}{24} + \frac{I}{96}\right) = \left(\frac{36}{24} + \frac{96}{96}\right) = 2.50" > 0.90" \therefore \text{ ok and } \frac{h}{12} = \frac{36}{12} = 3.0" > 0.90" \therefore \text{ ok}$$

<sup>&</sup>lt;sup>1</sup> Each end displacement was measured at the center of the support. Net displacement was the rail displacement relative to the supports.

<sup>&</sup>lt;sup>2</sup> Deflection limit calculation based on worse case 36" railing height to satisfy One- and Two-Family Dwelling requirements.





Series No. 1 (Continued)

Specimen No. 1 of 3 (Continued)

Test No. 4 - Test Date: 07/07/16  Design Load: 200 lb Concentrated Load at End of Top Rail (Bracket)						
Design	Load: 200 lb Co	ncentrated Load	at End of Top Rail (Bracket)			
Load Level Test Load E.T. (lb) (min:sec)			Result			
500 lb (2.50 x D.L.)	500 - 511	00:49 - 1:52	Withstood load equal to or greater than 500 lb for one full minute without failure			

Test No. 5 - 07/07/16  Design Load: 200 lb Concentrated Load at Top of Stand-Alone <sup>1</sup> Post (42 in high)						
Load Level	Test Load (lb)	E.T. (min:sec)	Post Displacement (in)			
Initial Load	40	00:00	0.00			
Design Load	214	00:09	0.77			
2.0x Design Load	401	00:23	2.36			
Initial Load	40	02:42	0.43			
	82% Recovery from 2.0 x Design Load					
2.5x Design Load	499 - 506	03:16 - 04:17	<b>Result</b> : Withstood load equal to or greater than 500 lb for one full minute without failure <sup>2</sup>			

### <u>Deflection Evaluation</u>:

Maximum post deflection at 214 lb = 0.77 in on a 42 in high post

Limits per AC273 <sup>3</sup>: 
$$\frac{h}{12} = \frac{36}{12} = 3.0$$
"> 0.77"  $\therefore ok$ 

<sup>&</sup>lt;sup>1</sup> Post was conservatively tested without a railing attached.

<sup>&</sup>lt;sup>2</sup> The test load fell below the target load for a total of 0.5 seconds throughout the duration of the load.

<sup>&</sup>lt;sup>3</sup> Deflection limit calculation based on worse case 36" railing height to satisfy One- and Two-Family Dwelling requirements.





Series No. 1 (Continued)

Specimen No. 2 of 3

Dosig	Test No. 1 - Test Date: 07/07/16  Design Load: 50 lb / 1 Square ft of In-Fill at Center of Two Pickets						
Load Level Test Load E.T. Result							
125 lb (2.50 x D.L.)	125 - 129	00:46 - 01:54	Sustained load equal to or greater than 125 lb for one full minute without failure				

Design	Test No. 2 - Test Date: 0707/16  Design Load: 50 lb / 1 Square ft of In-Fill at Bottom of Two Pickets						
Load Level Test Load E.T. (lb) (min:sec)			Result				
125 lb (2.50 x D.L.)	125 - 132	00:36 - 01:38	Sustained load equal to or greater than 125 lb for one full minute without failure				

Test No. 3 - Test Date: 07/07/16  Design Load: 200 lb Concentrated Load at Mid-Span of Top Rail						
Load Lovel	Test Load E.T. Displacement (in)					
Load Level	(lb)	(min:sec)	End	Mid	End	Net 1
200 lb (D.L.)	201	00:26	0.50	1.40	0.51	0.90
500 lb (2.50 x D.L.)	500 - 503	00:48 - 01:49	Result: Withstood load equal to or greater than 500 lb for one full minute without failure			

### **Deflection Evaluation:**

Maximum rail deflection at 201 lb = 0.90 in on an 8 ft rail

Limits per AC273 
$$^2$$
:  $\left(\frac{h}{24} + \frac{I}{96}\right) = \left(\frac{36}{24} + \frac{96}{96}\right) = 2.50$ " > 0.90 ∴ ok and  $\frac{h}{12} = \frac{36}{12} = 3.0$ " > 0.90" ∴ ok

<sup>&</sup>lt;sup>1</sup> Each end displacement was measured at the center of the support. Net displacement was the rail displacement relative to the supports.

<sup>&</sup>lt;sup>2</sup> Deflection limit calculation based on worse case 36" railing height to satisfy One- and Two-Family Dwelling requirements.





Series No. 1 (Continued)

Specimen No. 2 of 3 (Continued)

Test No. 4 - Test Date: 07/07/16  Design Load: 200 lb Concentrated Load at End of Top Rail (Bracket)				
Load Level	Test Load (lb)	E.T. (min:sec)	Result	
500 lb (2.50 x D.L.)	499 - 506	00:52 - 1:54	Withstood load equal to or greater than 500 lb for one full minute without failure 1	

<sup>&</sup>lt;sup>1</sup> The test load fell below the target load for a total of 0.5 seconds throughout the duration of the load.

Test No. 5 - 07/07/16  Design Load: 200 lb Concentrated Load at Top of Stand-Alone <sup>1</sup> Post (42 in high)					
Load Level	Test Load (lb)	E.T. (min:sec)	Post Displacement (in)		
Initial Load	39	00:00	0.00		
Design Load	201	00:06	0.76		
2.0x Design Load	401	00:31	2.43		
Initial Load	39	02:10	0.40		
	84% Rec	overy from 2.0 x	Design Load		
			Result: Withstood load equal to or		
2.5x Design Load	500 - 509	02:28 - 03:33	greater than 500 lb for one full minute		
			without failure		

### <u>Deflection Evaluation</u>:

Maximum post deflection at 201 lb = 0.76 in on a 42 in high post

Limits per AC273 <sup>2</sup>: 
$$\frac{h}{12} = \frac{36}{12} = 3.0$$
"> 0.76" .: ok

<sup>&</sup>lt;sup>1</sup> Post was conservatively tested without a railing attached.

<sup>&</sup>lt;sup>2</sup> Deflection limit calculation based on worse case 36" railing height to satisfy One- and Two-Family Dwelling requirements.





Series No. 1 (Continued)

Specimen No. 3 of 3

Test No. 1 - Test Date: 07/07/16  Design Load: 50 lb / 1 Square ft of In-Fill at Center of Two Pickets			
Load Level Test Load E.T. Result (Ib) (min:sec)			
125 lb (2.50 x D.L.)	125 - 129	01:19 - 01:26	Sustained load equal to or greater than 125 lb for one full minute without failure

Test No. 2 - Test Date: 07/07/16  Design Load: 50 lb / 1 Square ft of In-Fill at Bottom of Two Pickets				
Load Level Test Load (lb) E.T. (min:sec) Result				
125 lb (2.50 x D.L.)	125 - 128	00:16 - 01:22	Sustained load equal to or greater than 125 lb for one full minute without failure	

Test No. 3 - Test Date: 07/07/16  Design Load: 200 lb Concentrated Load at Mid-Span of Top Rail						
Load Level	Test Load E.T. Displacement (in)					
Load Level	(lb)	(min:sec)	End	Mid	End	Net 1
200 lb (D.L.)	200	00:34	0.52	1.39	0.46	0.90
500 lb (2.50 x D.L.)	501 - 507	00:45 - 01:53	Result: Withstood load equal to or greater than 500 lb for one full minute without failure			

### **Deflection Evaluation:**

Maximum rail deflection at 200 lb = 0.90 in on an 8 ft rail

Limits per AC273 
$$^2$$
:  $\left(\frac{h}{24} + \frac{I}{96}\right) = \left(\frac{36}{24} + \frac{96}{96}\right) = 2.5" > 0.90" \therefore ok \text{ and } \frac{h}{12} = \frac{36}{12} = 3.0" > 0.90" \therefore ok$ 

<sup>&</sup>lt;sup>1</sup> Each end displacement was measured at the center of the support. Net displacement was the rail displacement relative to the supports.

<sup>&</sup>lt;sup>2</sup> Deflection limit calculation based on worse case 36" railing height to satisfy One- and Two-Family Dwelling requirements.





Series No. 1 (Continued)

Specimen No. 3 of 3 (Continued)

Test No. 4 - Test Date: 07/07/16  Design Load: 200 lb Concentrated Load at End of Top Rail (Bracket)				
Load Level Test Load E.T. Result (lb) (min:sec)				
500 lb (2.50 x D.L.)	500 - 507	00:47 - 1:51	Withstood load equal to or greater than 500 lb for one full minute without failure	

Test No. 5 - 07/07/16  Design Load: 200 lb Concentrated Load at Top of Stand-Alone <sup>1</sup> Post (42 in high)					
Load Level	Test Load (lb) E.T. (min:sec)		Post Displacement (in)		
Initial Load	40	00:00	0.00		
Design Load	207	00:08	0.74		
2.0x Design Load	404	00:20	2.24		
Initial Load	45	01:58	0.36		
	84% Recove	ery from 2.0 x Des	ign Load		
2.5x Design Load	500 - 506	02:25 - 03:29	<b>Result</b> : Withstood load equal to or greater than 500 lb for one full minute without failure		

### <u>Deflection Evaluation</u>:

Maximum post deflection at 207 lb = 0.74 in on a 42 in high post

Limits per AC273 <sup>2</sup>: 
$$\frac{h}{12} = \frac{36}{12} = 3.0$$
" > 0.74" :: ok

<sup>&</sup>lt;sup>1</sup> Post was conservatively tested without a railing attached.

<sup>&</sup>lt;sup>2</sup> Deflection limit calculation based on worse case 36" railing height to satisfy One- and Two-Family Dwelling requirements.





## Series No. 2 72 in by 42 in *Lincoln* Railing with Structural Posts on Simulated Concrete Installation Limited to Use in IRC - One- and Two-Family Dwellings / ICC-ES™ AC273

### Specimen No. 1 of 3

	Test No. 1 - Test Date: 07/07/16				
Design I	Load: 50 lb / 1 Squ	uare ft of In-Fill at	Center of Two Pickets		
Load Level Test Load E.T. Result (lb)					
125 lb (2.50 x D.L.)	125 - 128	00:19 - 01:26	Sustained load equal to or greater than 125 lb for one full minute without failure		

Test No. 2 - Test Date: 07/07/16  Design Load: 50 lb / 1 Square ft of In-Fill at Bottom of Two Pickets				
Load Level Test Load (lb) E.T. (min:sec) Result				
125 lb (2.50 x D.L.)	125 - 129	00:21 - 01:29	Sustained load equal to or greater than 125 lb for one full minute without failure	

Test No. 3 - Test Date: 07/07/16  Design Load: 200 lb Concentrated Load at Mid-Span of Top Rail						
Lood Lovel	Test Load E.T. Displacement (in)					
Load Level	(lb)	(min:sec)	End	Mid	End	Net 1
200 lb (D.L.)	200	00:31	0.46	0.85	0.48	0.38
500 lb (2.50 x D.L.)	501 - 507	00:46 - 01:53	Result: Withstood load equal to or greater than 500 lb for one full minute without failure			

### **Deflection Evaluation:**

Maximum rail deflection at 200 lb = 0.38 in on a 6 ft rail

Limits per AC273 <sup>2</sup>: 
$$\left(\frac{h}{24} + \frac{l}{96}\right) = \left(\frac{36}{24} + \frac{72}{96}\right) = 2.25'' > 0.38'' \therefore \text{ ok and } \frac{h}{12} = \frac{36}{12} = 3.0'' > 0.38'' \therefore \text{ ok}$$

<sup>&</sup>lt;sup>1</sup> Each end displacement was measured at the center of the support. Net displacement was the rail displacement relative to the supports.

<sup>&</sup>lt;sup>2</sup> Deflection limit calculation based on worse case 36" railing height to satisfy One- and Two-Family Dwelling requirements





Series No. 2 (Continued)

Specimen No. 1 of 3 (Continued)

	Test No. 4 - Test Date: 07/07/16				
Design	Design Load: 200 lb Concentrated Load at End of Top Rail (Bracket)				
Load Level	Load Level Test Load E.T. Result (Ib) (min:sec)				
500 lb (2.50 x D.L.)	499 - 505	01:00 - 02:06	Withstood load equal to or greater than 500 lb for one full minute without failure		

<sup>&</sup>lt;sup>1</sup> The test load fell below the target load for a total of 1.5 seconds throughout the duration of the load.

Test No. 5- Test Date: 07/07/16  Design Load: 200 lb Concentrated Load at Top of Post Mount (42 in High)					
Load Level	Load Level Test Load E.T. Displacement (in)				
200 lb (D.L.)	204	00:10	0.96		
500 lb (2.50 x D.L.)	500 - 509	00:35 - 01:42	<b>Result</b> : Withstood load equal to or greater than 500 lb for one full minute without failure		

### **Deflection Evaluation:**

Maximum post deflection at 204 lb = 0.96 in on a 6 ft rail

Limits per AC273 <sup>1</sup>: 
$$\frac{h}{12} = \frac{36}{12} = 3.00" > 0.96" \therefore ok$$

<sup>&</sup>lt;sup>1</sup> Deflection limit calculation based on worse case 36" railing height to satisfy One- and Two-Family Dwelling requirements.





### Series No. 2 (Continued)

### Specimen No. 2 of 3

Test No. 1 - Test Date: 07/08/16  Design Load: 50 lb / 1 Square ft of In-Fill at Center of Two Pickets					
Load Level	Load Level Test Load E.T. Result (Ib)				
125 lb (2.50 x D.L.)	125 - 127	00:12 - 01:19	Sustained load equal to or greater than 125 lb for one full minute without failure		

Test No. 2 - Test Date: 07/08/16  Design Load: 50 lb / 1 Square ft of In-Fill at Bottom of Two Pickets				
Load Level Test Load E.T. Result (Ib) (min:sec)				
125 lb (2.50 x D.L.)	125 - 128	00:20 - 01:28	Sustained load equal to or greater than 125 lb for one full minute without failure	

Test No. 3 - Test Date: 07/08/16 Design Load: 200 lb Concentrated Load at Mid-Span of Top Rail						
Load Level	Test Load F.T. Displacement (in)					
Load Level	(lb)	(min:sec)	End	Mid	End	Net 1
200 lb (D.L.)	201	00:53	0.60	1.06	0.73	0.40
500 lb (2.50 x D.L.)	499 - 504	Result: Withstood load equal to or 00:18 - 02:15 greater than 500 lb for one full minute without failure <sup>2</sup>				

### **Deflection Evaluation:**

Maximum rail deflection at 201 lb = 0.40 in on a 6 ft rail

Limits per AC273 <sup>3</sup>: 
$$\left(\frac{h}{24} + \frac{I}{96}\right) = \left(\frac{36}{24} + \frac{72}{96}\right) = 2.25" > 0.40" : ok and  $\frac{h}{12} = \frac{36}{12} = 3.0" > 0.40" : ok$$$

<sup>&</sup>lt;sup>1</sup> Each end displacement was measured at the center of the support. Net displacement was the rail displacement relative to the supports.

<sup>&</sup>lt;sup>2</sup> The test load fell below the target load for a total of 1.5 seconds throughout the duration of the load.

<sup>&</sup>lt;sup>3</sup> Deflection limit calculation based on worse case 36" railing height to satisfy One- and Two-Family Dwelling requirements.





Series No. 2 (Continued)

Specimen No. 2 of 3 (Continued)

Design	Test No. 4 - Test Date: 07/08/16  Design Load: 200 lb Concentrated Load at End of Top Rail (Bracket)				
Load Level	Load Level Test Load E.T. Result (lb) (min:sec)				
500 lb (2.50 x D.L.)	Withstood load equal to or greater 500 - 507 00:47 - 01:54 than 500 lb for one full minute without failure				

Design Loa	Test No. 5 - Test Date: 07/08/16 Design Load: 200 lb Concentrated Load at Top of Post Mount (42 in High)					
Load Level Test Load E.T. Displacement (in)						
200 lb (D.L.)	204	00:15	1.20			
500 lb (2.50 x D.L.)	500 - 506	00:46 - 01:52	Result: Withstood load equal to or greater than 500 lb for one full minute without failure			

### **Deflection Evaluation:**

Maximum post deflection at 204 lb = 1.20 in on a 42 in high post

Limits per AC273 <sup>1</sup>: 
$$\frac{h}{12} = \frac{36}{12} = 3.00'' > 1.20'' \therefore ok$$

<sup>&</sup>lt;sup>1</sup> Deflection limit calculation based on worse case 36" railing height to satisfy One- and Two-Family Dwelling requirements.





### Series No. 2 (Continued)

Specimen No. 3 of 3

Desig	Test No. 1 - Test Date: 07/08/16  Design Load: 50 lb / 1 Square ft of In-Fill at Center of Two Pickets				
Load Level	Load Level Test Load E.T. Result				
125 lb (2.50 x D.L.)	Sustained load equal to or greater  125 lb  125 - 131  00:16 - 01:24  than 125 lb for one full minute				

Test No. 2 - Test Date: 07/08/16  Design Load: 50 lb / 1 Square ft of In-Fill at Bottom of Two Pickets					
Load Level	Load Level Test Load E.T. Result				
125 lb (2.50 x D.L.)	127 - 135	00:12 - 01:20	Sustained load equal to or greater than 125 lb for one full minute without failure		

Test No. 3 - Test Date: 07/08/16  Design Load: 200 lb Concentrated Load at Mid-Span of Top Rail						
Lood Lovel	Test Load E.T. Displacement (in)					
Load Level	(lb)	(min:sec)	End	Mid	End	Net 1
200 lb (D.L.)	201	00:22	0.53	0.97	0.59	0.41
500 lb (2.50 x D.L.)	500 - 504	Result: Withstood load equal to or 00:38 - 01:46 greater than 500 lb for one full minute without failure				

### <u>Deflection Evaluation</u>:

Maximum rail deflection at 201 lb = 0.41 in on a 6 ft rail

Limits per AC273 <sup>2</sup>: 
$$\left(\frac{h}{24} + \frac{l}{96}\right) = \left(\frac{36}{24} + \frac{72}{96}\right) = 2.25" > 0.41" : ok and  $\frac{h}{12} = \frac{36}{12} = 3.0" > 0.41" : ok$$$

<sup>&</sup>lt;sup>1</sup> Each end displacement was measured at the center of the support. Net displacement was the rail displacement relative to the supports.

<sup>&</sup>lt;sup>2</sup> Deflection limit calculation based on worse case 36" railing height to satisfy One- and Two-Family Dwelling requirements





Series No. 2 (Continued)

Specimen No. 3 of 3 (Continued)

Test No. 4 - Test Date: 07/08/16  Design Load: 200 lb Concentrated Load at End of Top Rail (Bracket)				
Load Level Test Load E.T. Result (lb) (min:sec)				
500 lb (2.50 x D.L.)	500 - 508 00:40 - 01:44 Withstood load equal to or greater than 500 lb for one full minute without failure			

Test No. 5 - Test Date: 07/08/16  Design Load: 200 lb Concentrated Load at Top of Post Mount (42 in High)					
Load Level Test Load E.T. Displacement (in)					
200 lb (D.L.)	203	00:13	1.19		
500 lb (2.50 x D.L.)	499 - 508	00:49 - 01:57	<b>Result</b> : Withstood load equal to or greater than 500 lb for one full minute without failure		

### **Deflection Evaluation:**

Maximum post deflection at 203 lb = 1.19 in on a 42 in high post

Limits per AC273 <sup>2</sup>: 
$$\frac{h}{12} = \frac{36}{12} = 3.00" > 1.19" : ok$$

<sup>&</sup>lt;sup>1</sup> The test load fell below the target load for a total of 0.5 seconds throughout the duration of the load.

<sup>&</sup>lt;sup>2</sup> Deflection limit calculation based on worse case 36" railing height to satisfy One- and Two-Family Dwelling requirements.





### 5.8 Summary and Conclusions

When installed between adequate supports, the railing assemblies reported herein meet the structural performance requirements of Section 4.2.1 of ICC-ES™ AC273 with guardrail details and Occupancy Classification as shown in the following table:

Lincoln Aluminum Railing Systems						
Configuration	Guardrail Size	Post Installation	Code Occupancy Classification			
Level / In-Line	96 in x 42 in	Surface Mount Wood	IRC – One- and Two-			
Level / In-Line	72 in x 42 in	or Simulated Concrete	Family Dwellings			

Anchorage of support posts to the supporting structure is not included in the scope of this testing and would need to be evaluated separately.

### 6.0 Closing Statement

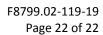
Intertek-ATI will service this report for the entire test record retention period. Test records that are retained such as detailed drawings, datasheets, representative samples of test specimens, or other pertinent project documentation will be retained by Intertek-ATI for the entire test record retention period.

Results obtained are tested values and were secured using the designated test methods. This report does not constitute certification of this product nor an opinion or endorsement by this laboratory. It is the exclusive property of the client so named herein and relates only to the specimens tested. This report may not be reproduced, except in full, without the written approval of Intertek-ATI.

For INTERTEK-ATI:	
Emily C. Riley Project Manager ECR:vtm/jas	V. Thomas Mickley, Jr., P.E. Senior Project Engineer

Attachments (pages): This report is complete only when all attachments listed are included.

Appendix A - Drawings (14) Appendix B - Photographs (7)







### **Revision Log**

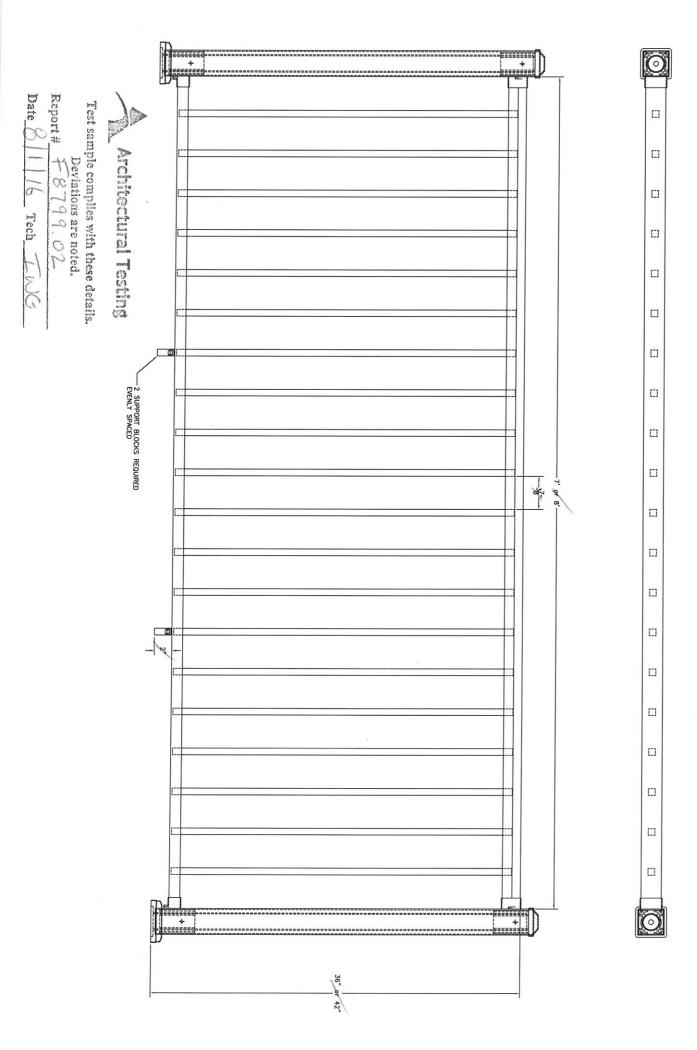
<u>Rev. #</u>	<u>Date</u>	Page(s)	Revision(s)
0	09/02/16	N/A	Original report issue

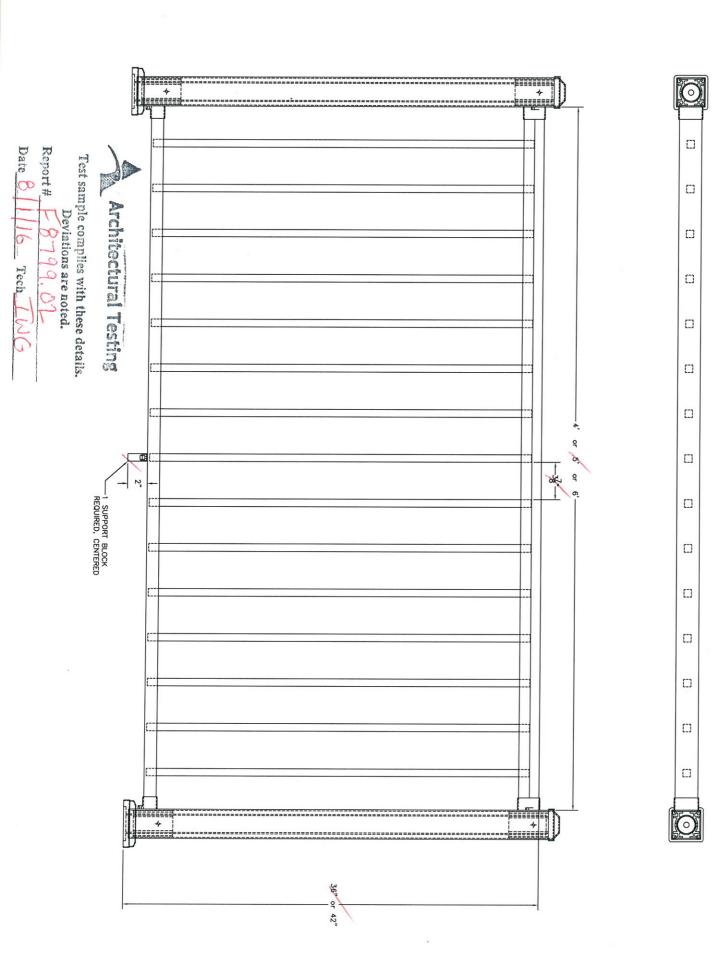


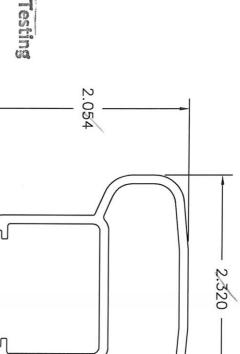


### **APPENDIX A**

**Drawings** 







## Architectural Testing

Test sample complies with these details.

Deviations are noted.

Date 8 Report # 6 Tech ING

MATERIAL 6063 T5 ALUMINUM

830027	820027	810027	800027 810027		780027	84"
830025	820025	810025	800025	790025	920082 220082	60"
830024	820024	810024	800024	790024	780024	48"
CLAY	TAN	BRONZE	ATIHM	BLACK	Length No Paint BLACK	Length

DRAWER 300 SHEBOYGAN FALLS 'H. (920) 467—4685 FAX. (920	0	TOU
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SHEB 7-468	CUSTOM EXTRUSIONS	_
OYGAN 35 FAX	EXT	17.
( (92)	<b>Sign</b>	נוניטו
S, WI	S	· ·
FALLS, WI 53085 (920) 467-3271		ò.

WALL					
AREA-			LOG		
FLEXIBLE	Æ		REV.		
RIGID			DR. BY	HO	
TOLER	TOLERANCES-		SCALE	FULL	
×			DATE	1/15/14	
XXX	#		DIE		
ANGLES	S				
TOP RAIL	AIL				
PART NO.	SEE	CHART	RT		

120"	96"	84.	72"	60"	8.	Length
780070	780022	780021	780020		780018	No Point
790070	790022	790021	790020	790019	790018	BLACK
800070	800022		800020			ЭЛНМ
810070	810022	810021	810020	810019	810018	BRONZE
820070	820022		820020		820018	TAN
830070	830022	830021	830020	830019	830018	CLAY

# STRAIGHT SECTIONS





Report # Test sample complies with these details.

Deviations are noted.

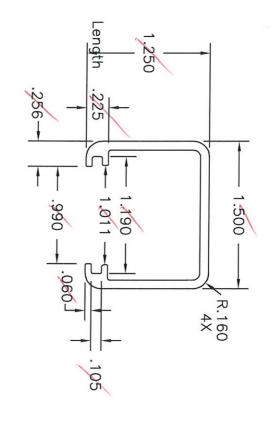
Report # F8799.02

Tech LWG

- 3.45 - 72.60 - 72.60 2.515 2.515 2.515 2.515 2.515 2.515 2.515 2.515 2.515 2.515 2.515 -	'SE	7,	S	σį
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## Architectural Testing

MATERIAL 6063 T5 ALUMINUM

Test sample complies with these details.

Deviations are noted.

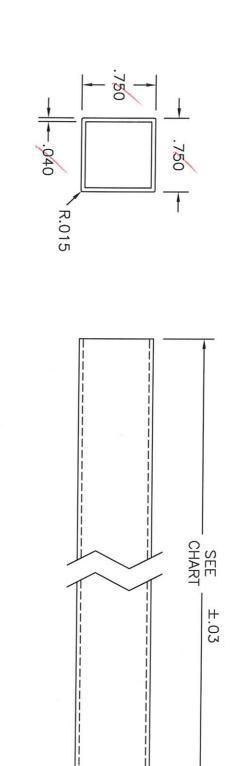
Report # F8799.02

Date 8/1/16 Tech TWG

120	100"	96"	84"	72"	60"	48"	Length
,0000	780068	780016	780015	780014	780013	780012	Length No Paint BLACK
	830007	790016	790015	790014	790013	790012	BLACK
000000		800016 810016	800015	800014 810014	790013 800013 810013	800012 810012	WHITE
010000	010060	810016	810015	810014	810013	810012	BRONZE
84008		820016 830016	820015	820014	820013	820012	TAN
830068	22000	830016	830015	830014	830013	830012	CLAY

_	_				_			_	_			
BOTTOM RAIL	ANGLES	XXX ±	×	TOLERANCES-	RIGID	FLEXIBLE	AREA-	WALL .090	PH. (920) 467-4685 FAX. (920) 467-3271	DRAWER 300 SHEBOYGAN FALLS,	CŪSTOM EXTRUSIONS	FOLY VY
		DIE	DATE 2/3/14	SCALE FULL	DR. BY DH	REV.	LOG		FAX. (920) 467-3271	GAN FALLS, WI 53085	XTRUSIONS	inil co.

PART SEE CHART





## Architectural Testing

MATERIAL 6063 T5 ALUMINUM

Test sample complies with these details.

Deviations are noted.

Report # F8799,02

Report #

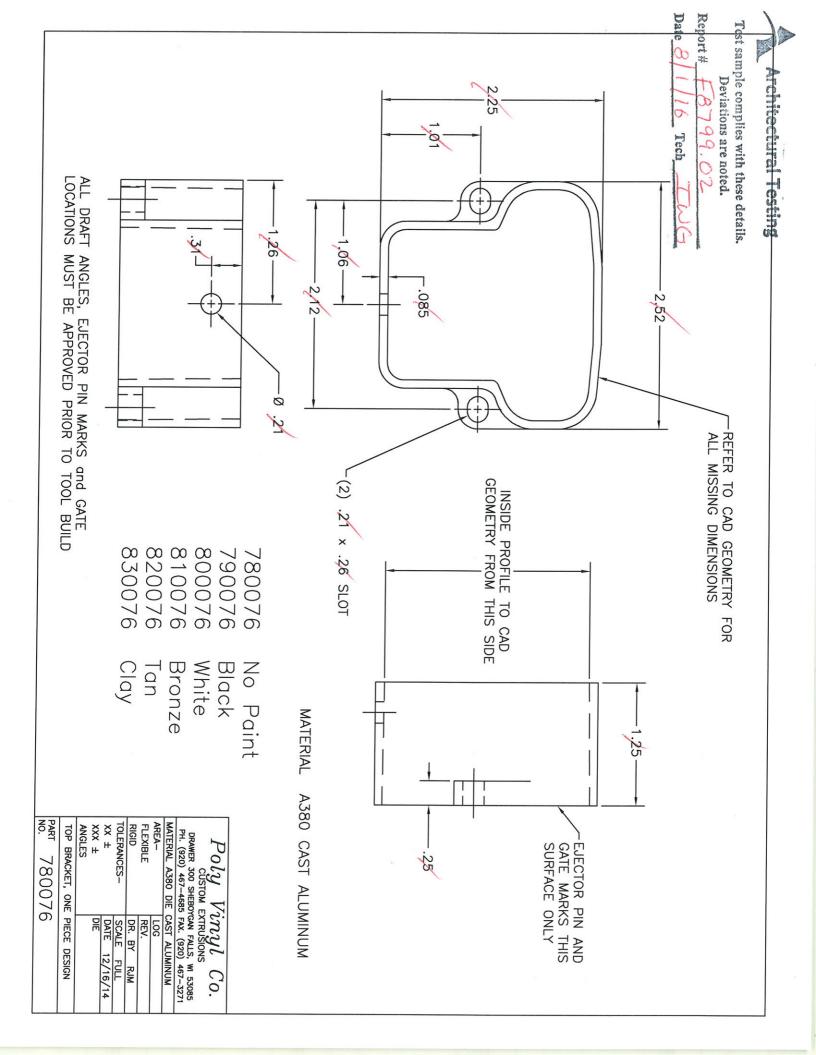
6 Tech

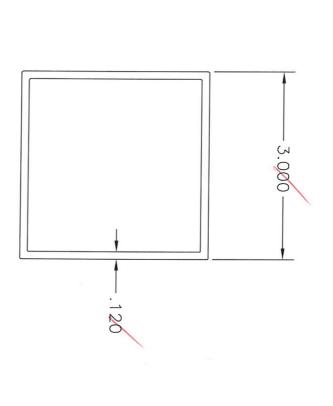
2.00	6.50	14.00	31.00	33.00	39.00	Length
780000	780004	780048	780003	780001	780002	Length No Paint BLACK
790000	790004	790048	790003	790001	790002	BLACK
N/A	800004	N/A	800003	800001	800002	WHITE
N/A	810004	N/A	810003	810001	810002	BRONZE
N/A	820004	N/A	820003	820001	820002	TAN
N/A	830004	N/A	830003	830001	830002	CLAY

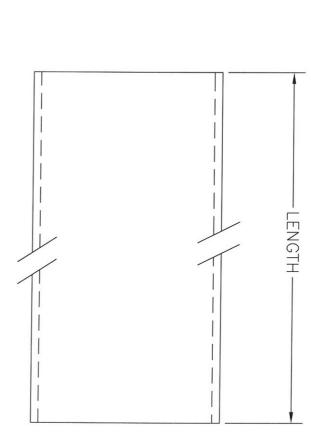
CUSTOM EXTRUSIONS 300 SHEBOYGAN FALLS, WI 53085 467-4685 FAX. (920) 467-3271 LOG REV. DR. BY DH SCALE FULL STATE 1 732 74	CUSTOM EXTRUSIONS DRAWER 300 SHEBOYGAN FALLS, PH. (920) 467–4685 FAX. (920) WALL AREA FLEXIBLE RIGID TOLERANCES TOLERANCE
---	--

ANGLES Balusters

PART SEE CHART









Test sample complies with these details.

Deviations are noted.

Report # F8799.02

Report #

Date 81 16 Tech \_\_\_\_

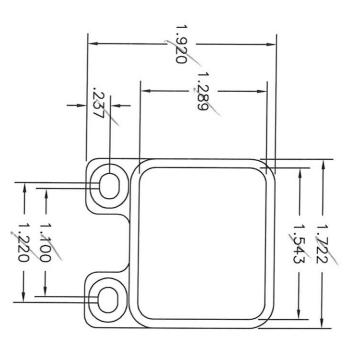
 
 Length
 No
 Paint
 BLACK
 WHITE
 BRONZE
 TAN

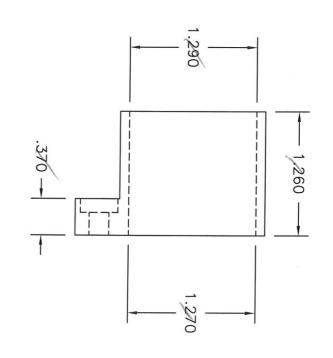
 37.25
 780127
 790127
 800127
 810127
 820127
 780105 | 790105 | 800105 | 810105 | 820105 | 830105 CLAY 830127

43.25

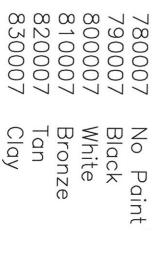
MATERIAL 6063 T5 ALUMINUM

PART	POST EXTRUSION,	ANGLES	XXX ±	×	TOLERANCES-	RICID	FLEXIBLE	AREA- 0	WALL	PH. (920) 467-4685 FAX. (920)	CUSTOM	Poly V
	6063 15		DIE	DATE	SCALE	DR. BY	REV.	L06		FAX. (920	EXTRUSION	Vinyl
				2/22/16	FULL	RJM				WI 53085	S	$C_0$ .





# MATERIAL A380 CAST ALUMINUM





Test sample complies with these details.

Deviations are noted.

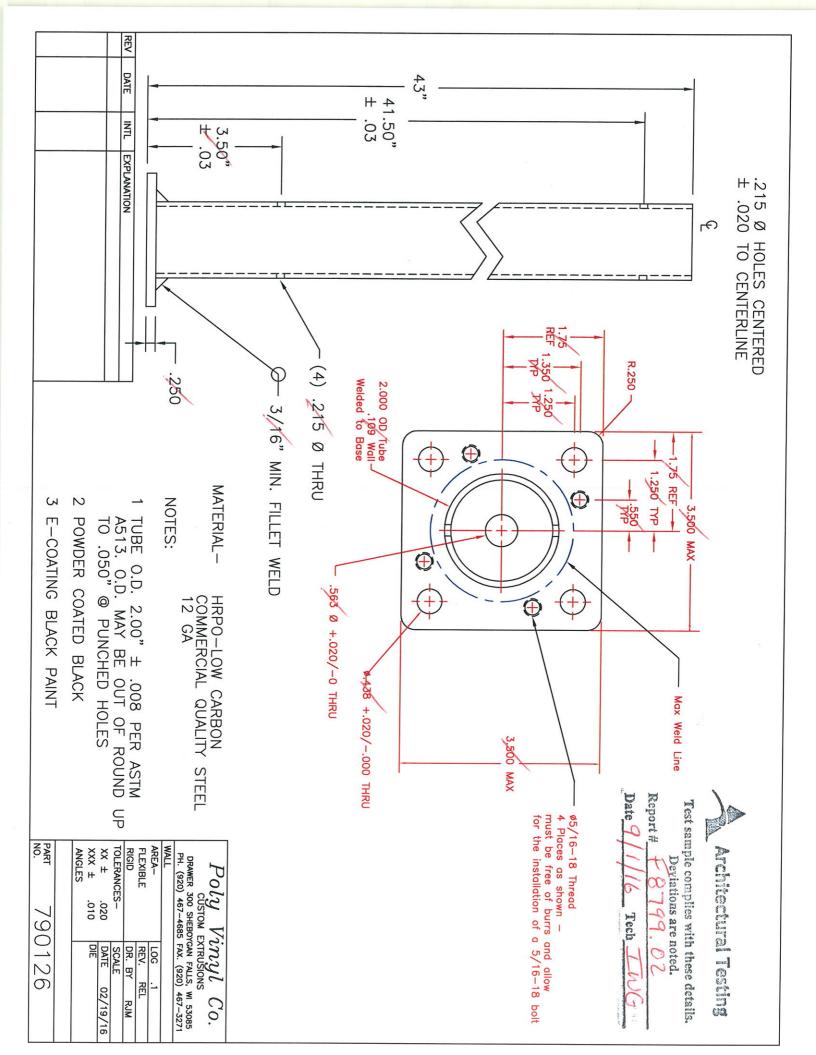
Report # F8799,02

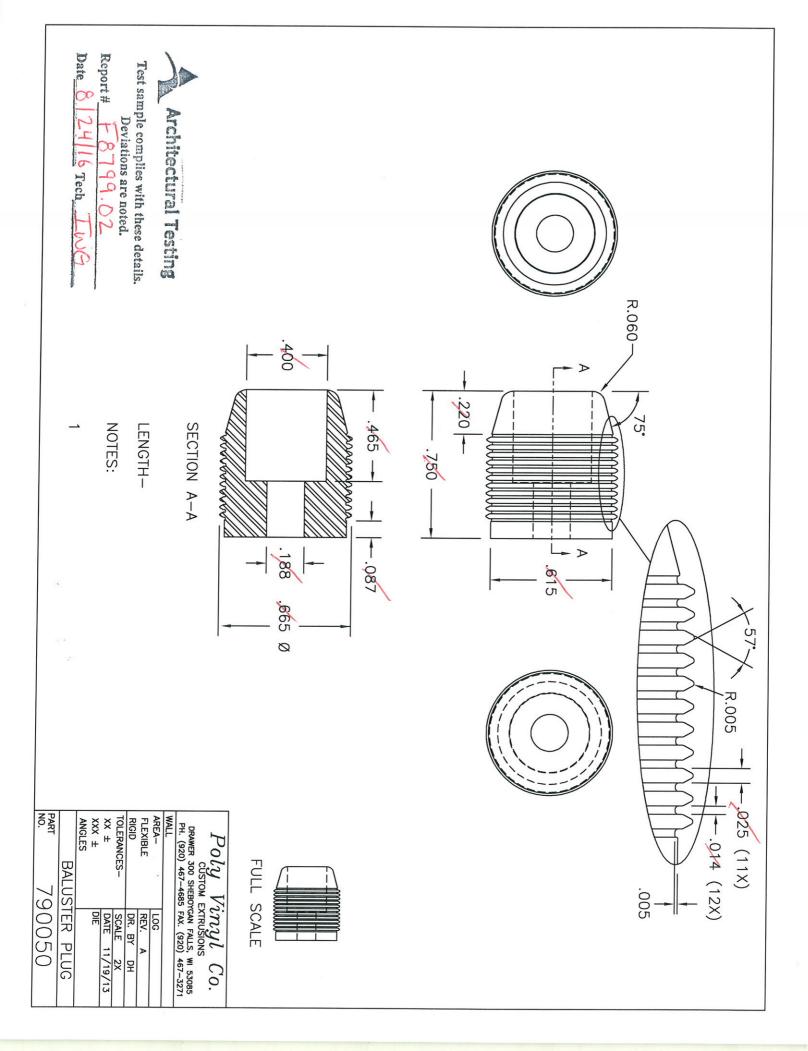
Date 8

Tech

	WALL
467-3271	PH. (920) 467-4685 FAX. (920) 467-3271
WI 53085	DRAWER 300 SHEBOYGAN FALLS, WI 53085
-	CUSTOM EXTRUSIONS
CO.	Foly Vinyl
2	7) 7

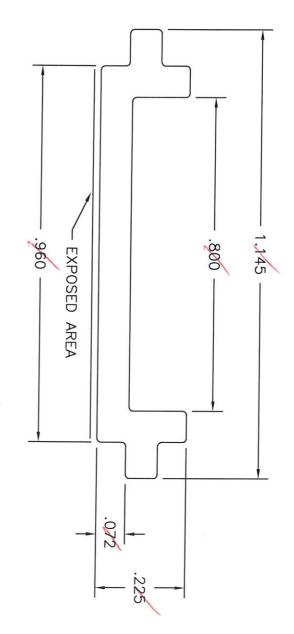
PART 780007	BOTTOM BRACKET	ANGLES	XXX ±	×	TOLERANCES-	RIGID	FLEXIBLE	AREA-	אארר
			DIE	DATE 8/28/13	SCALE FULL	DR. BY DH	REV.	LOG	





BREAK ALL CORNERS AT R.010

ACTUAL SIZE





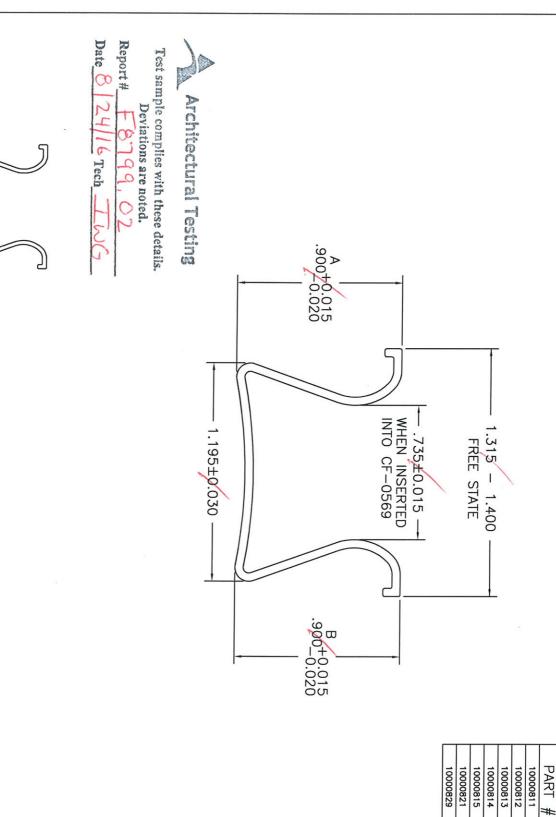
Test sample complies with these details.

Deviations are noted.

Report # F 8799.02

Date 8 24/16 Tech ING

	5000000												
PART NO.	RAIL INSERT E	ANGLES	XXX ±	××	TOLERANCES-	RIGID	FLEXIBLE	AREA115	WALL .080	DRAWER 300 SHEBOYGAN FALLS, PH. (920) 467-4685 FAX. (920)	CUSTOM EXTRUSIONS	Poly Va	1
	EXTRUSION		DIE	DATE 2/3/14	SCALE 4X	DR. BY DH	REV.	LOG		YGAN FALLS, WI 53085 FAX. (920) 467-3271	S	Vinyl Co.	



LENGTH ± .25

47.50

59.50 71.50 83.50 95.50 119.50 35.50

PROCESS GUIDE

2

3855

AREA-FLEXIBLE RIGID

.152

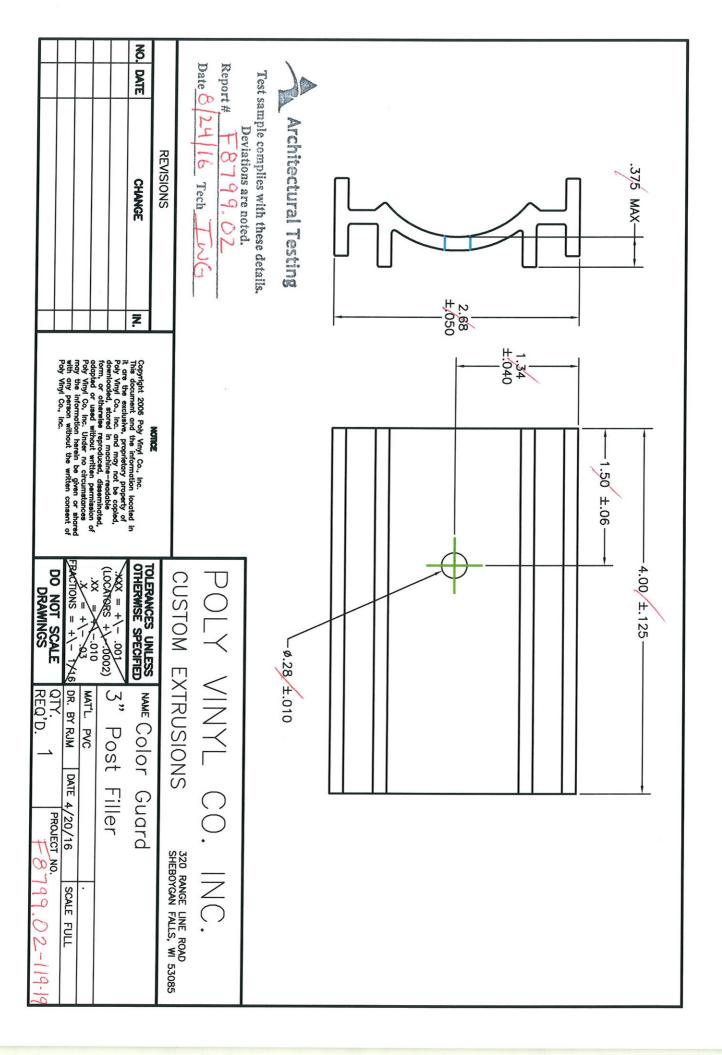
REV.

NOTES:

Poly Vinyl Co.

CUSTOM EXTRUSIONS

DRAWER 300 SHEBOYGAN FALLS, WI 53085
PH. (920) 467–4885 FAX. (920) 467–3271







### **APPENDIX B**

### **Photographs**





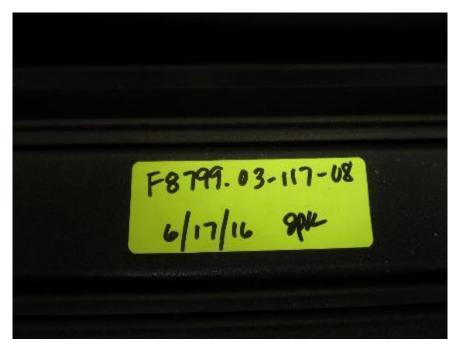


Photo No. 1
Typical Sampling Mark



Photo No. 2
Typical ASTM E 8 Tensile Test Setup







Photo No. 3
Assembly Fastener Test Setup







Photo No. 4
In-Fill Load Test at Center of Two Pickets



Photo No. 5
In-Fill Load Test at Bottom of Two Pickets







Photo No. 6
Concentrated Load Test at Mid-Span of Top Rail







Photo No. 7
Concentrated Load Test at Rail End (Bracket)



Photo No. 8
Concentrated Load Test at Top of Post Mount
(Surface Mount Wood Installation)







Photo No. 9
Concentrated Load Test at Top of Post Mount
(Simulated Concrete Installation)

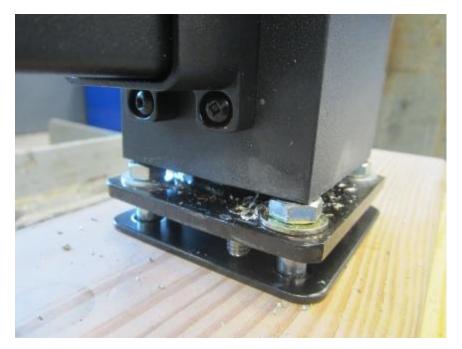


Photo No. 10
Bottom Rail Bracket and Post Mount Connection





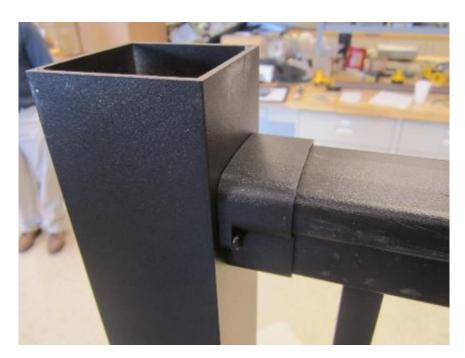


Photo No. 11 Top Rail Bracket Connection